

Karlsruhe Institute of Technology



# Finite deformation phase-field approach to fracture

C. Hesch and K. Weinberg

#### Introduction

Fracture mechanics [1]

- Classical brittle fracture approaches of Griffith and Irwin
- Material fails locally upon a specific fracture energy

## **Consistent discretisation**

- Conform discretisation of the fields  $\varphi^{h} = \sum_{A \in \omega} N^{A}(\mathbf{X}) \mathbf{q}_{A}, \quad \delta \varphi^{h} = \sum_{A \in \omega} N^{A}(\mathbf{X}) \delta \mathbf{q}_{A}$   $\varphi^{h} = \sum_{A \in \omega} N^{A}(\mathbf{X}) \varphi_{A}, \quad \delta \varphi^{h} = \sum_{A \in \omega} N^{A}(\mathbf{X}) \delta \varphi_{A}$
- Continuum formulation solve via standard finite-element strategies

Thermodynamically consistent approach [2]

- Finite deformation approach multiplicative decomposition of the local deformation
- Enables the use of phase-field models for general nonlinear constitutive laws
- Energy-momentum consistent time integration scheme

# **Field equations**

Phase field equation

 $\mathfrak{s} - l^2 \Delta \mathfrak{s} = 0 \text{ in } \mathcal{B}_0$  $\nabla \mathfrak{s} \cdot \boldsymbol{n} = 0 \text{ on } \delta \mathcal{B}_0$ 

Regularized crack surface topology

$$\Gamma_{I}(\mathfrak{s}) := \int_{\mathcal{B}_{0}} \gamma_{I}(\mathfrak{s}) \, \mathrm{d}V = \int_{\mathcal{B}_{0}} \frac{1}{2I} \mathfrak{s}^{2} + \frac{1}{2} \nabla(\mathfrak{s}) \cdot \nabla(\mathfrak{s}) \, \mathrm{d}V$$

Local balance of linear momentum

$$\dot{\boldsymbol{\varphi}} = \boldsymbol{v}, \, 
ho_0 \dot{\boldsymbol{v}} = \mathsf{Div}\, \boldsymbol{P} + \bar{\boldsymbol{B}}$$

$$\mathfrak{s} = \sum_{A \in \omega} \mathcal{N} (\Lambda) \mathfrak{s}_{A}, \quad \mathfrak{o}\mathfrak{s} = \sum_{A \in \omega} \mathcal{N} (\Lambda) \mathfrak{o}\mathfrak{s}_{A}$$

Discrete phase field

$$\delta \mathfrak{s}_{A} \left[ \int_{\mathcal{B}_{0}} \frac{g_{c}}{I} N^{A} N^{B} \mathfrak{s}_{B} + g_{c} I \nabla N^{A} \cdot \nabla N^{B} \mathfrak{s}_{B} + \frac{\partial \Psi}{\partial \mathfrak{s}_{A}} \, \mathrm{d} \, V \right] = 0$$

Full discrete system

$$\delta \boldsymbol{q}_{A} \cdot \left[ \boldsymbol{M}^{AB} \frac{\boldsymbol{v}_{B,n+1} - \boldsymbol{v}_{B,n}}{\Delta t} + \int_{\mathcal{B}_{0}} \nabla \boldsymbol{N}^{A}(\boldsymbol{X}) \cdot \boldsymbol{S}^{e,h}_{n,n+1} \nabla \boldsymbol{N}^{B}(\boldsymbol{X}) \, \mathrm{d} \boldsymbol{V} \boldsymbol{q}_{B,n+1/2} \right] = \delta \boldsymbol{q}_{A} \cdot \left[ \boldsymbol{F}^{A,ext}_{n+1/2} \right]$$
$$\delta \mathfrak{s}_{A} \left[ \int_{\mathcal{B}_{0}} \frac{g_{c}}{l} \boldsymbol{N}^{A} \boldsymbol{N}^{B} \mathfrak{s}_{B,n+1/2} + g_{c} l \nabla \boldsymbol{N}^{A} \cdot \nabla \boldsymbol{N}^{B} \mathfrak{s}_{B,n+1/2} \, \mathrm{d} \boldsymbol{V} \right] = -\delta \mathfrak{s}_{A} \int_{\mathcal{B}_{0}} \left( \frac{\partial \Psi}{\partial \mathfrak{s}_{A}} \right)_{n+1/2} \, \mathrm{d} \boldsymbol{V}$$

$$\boldsymbol{S}_{n,n+1}^{e,h} = 2 \frac{\partial \Psi_{n+1/2}}{\partial \boldsymbol{C}^{h}} + 2 \frac{\Psi_{n+1} - \Psi_{n} + g_{c}(\gamma_{n+1} - \gamma_{n}) - \frac{\partial \Psi_{n+1/2}}{\partial \boldsymbol{C}^{h}} : \Delta \boldsymbol{C}^{h}}{\Delta \boldsymbol{C}^{h}} : \Delta \boldsymbol{C}^{h}$$

## Numerical example

Pure shear test - uniform mesh with 256  $\times$  256 elements

- $\boldsymbol{\varphi} = \bar{\boldsymbol{\varphi}} \text{ on } \partial \mathcal{B}_0^{\varphi} \times \mathcal{I}, \ \boldsymbol{PN} = \bar{\boldsymbol{T}} \text{ on } \partial \mathcal{B}_0^{\sigma} \times \mathcal{I}$
- Weak form of the nonlinear problem

$$\int_{\mathcal{B}_0} \delta \boldsymbol{\varphi} \cdot \rho_0 \dot{\boldsymbol{v}} \, \mathrm{d}\boldsymbol{V} + \int_{\mathcal{B}_0} \boldsymbol{S} : \boldsymbol{F}^T \nabla_{\boldsymbol{X}} (\delta \boldsymbol{\varphi}) \, \mathrm{d}\boldsymbol{V} = \int_{\mathcal{B}_0} \delta \boldsymbol{\varphi} \cdot \boldsymbol{\bar{B}} \, \mathrm{d}\boldsymbol{V} + \int_{\partial \mathcal{B}_0^{\sigma}} \delta \boldsymbol{\varphi} \cdot \boldsymbol{\bar{T}} \, \mathrm{d}\boldsymbol{A}$$

## **Operator split**

Decomposition into compressive and tensile part

$$m{F} = m{F}^- m{F}^+ = \sum_{a=1}^n \lambda_a^- \lambda_a^+ m{n}_a \otimes m{N}_a$$

Further decomposition of tensile stretches

$$m{F} = \sum_{a=1}^{n} (\lambda_a^+)^{\mathfrak{s}} (\lambda_a^+)^{(1-\mathfrak{s})} \lambda_a^- m{n}_a \otimes m{N}_a$$

Elastic, fracture insensitive part

$$\boldsymbol{F}^{e} = \sum_{a=1}^{n} (\lambda_{a}^{+})^{(1-\mathfrak{s})} \lambda_{a}^{-} \boldsymbol{n}_{a} \otimes \boldsymbol{N}_{a}, \ \boldsymbol{C}^{e} = (\boldsymbol{F}^{e})^{T} \boldsymbol{F}^{e}$$

Corresponding second Piola–Kirchhoff stress tensor



Phase-field patterns for non-linear shear test. Upper row at a displacement of  $u = [9.0, 11.0, 13.3] \times 10^{-3}$  mm for a length scale of 0.0150 mm. Lower row at a displacement of  $u = [9.0, 11.0, 15.1] \times 10^{-6}$  mm for a length scale of 0.0075 mm.

## References



• Link to linear theory:  $\epsilon_a = \log(\lambda_a)$ 

$$\boldsymbol{\epsilon}^{\mathsf{e}} = \sum_{a=1}^{n} \left( (\mathbf{1} - \boldsymbol{\mathfrak{s}}) \boldsymbol{\epsilon}_{a}^{+} \boldsymbol{N}_{a} \otimes \boldsymbol{N}_{a} + \boldsymbol{\epsilon}_{a}^{-} \boldsymbol{N}_{a} \otimes \boldsymbol{N}_{a} \right)$$

C. Miehe, M. Hofacker and F. Welschinger Thermodynamically consistent phase-field models of fracture: Variational principles and multi-field FE implementations. *Int. J. Numer. Meth. Engng*, 83:1273–1311, 2010.

#### C. Hesch and K. Weinberg

Thermodynamically consistent algorithms for a finite-deformation phase-field approach to fracture

*Int. J. Numer. Meth. Engng*, DOI: 10.1002/nme.4709

KIT – University of the State of Baden-Wuerttemberg and National Laboratory of the Helmholtz Association

PD Dr.-Ing. habil. Christian Hesch