

Energy-momentum schemes for large deformation contact problems

Christian Hesch^{*1} und P. Betsch^{**1}

¹ Universität Siegen, Paul-Bonatz-Str. 9-11, 57068 Siegen, Germany.

Dynamic contact problems in elasticity are dealt with in the framework of nonlinear finite element methods. A new energy-momentum conserving time-stepping scheme for the mortar contact formulation is presented. The proposed method relies on a reparametrization of the contact constraints in terms of specific invariants. For the time discretisation of the contact forces emanating from the mortar formulation the notion of a discrete gradient is applied.

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1 Hamiltonian formulation of discrete elastodynamics

We start with the space finite element discretisation of nonlinear elastodynamics, which gives rise to a discrete strain energy function, given by

$$V^{\text{int}}(\mathbf{q}) = \int_{\mathcal{B}} W(\mathbf{C}) \, dV \quad (1)$$

where \mathbf{q} represents a possible discrete configuration and \mathbf{C} the discrete version of the deformation tensor (right Cauchy-Green tensor). Furthermore, we assume, that the external forces can be derived from an energy potential

$$V^{\text{ext}}(\mathbf{q}) = - \int_{\mathcal{B}} \rho_0 \mathbf{b} \cdot \boldsymbol{\varphi} \, dV - \int_{\partial \mathcal{B}_\sigma} \bar{\mathbf{t}} \cdot \boldsymbol{\varphi} \, dA \quad (2)$$

where ρ_0 denotes the reference mass density, \mathbf{b} the applied body force, $\boldsymbol{\varphi}$ the actual configuration and $\bar{\mathbf{t}}$ the prescribed traction boundary condition. Due to the presence of contact constraints, the equations of motion pertaining to the fully discrete system can be written by using a mid-point-type discretisation of the underlying system of differential algebraic equations:

$$\begin{aligned} \mathbf{q}_{n+1} - \mathbf{q}_n &= \frac{\Delta t}{2} (\mathbf{v}_n + \mathbf{v}_{n+1}) \\ \mathbf{M}(\mathbf{v}_{n+1} - \mathbf{v}_n) &= -\Delta t \bar{\nabla}_{\mathbf{q}} V(\mathbf{q}_n, \mathbf{q}_{n+1}) - \Delta t \sum_{l=1}^m (\lambda_l)_{n+1} \bar{\nabla}_{\mathbf{q}} \Phi_l(\mathbf{q}_n, \mathbf{q}_{n+1}) \\ \mathbf{0} &= \Phi(\mathbf{q}_{n+1}) \end{aligned} \quad (3)$$

Here, the discrete gradient is defined as

$$\bar{\nabla}_{\mathbf{q}} V(\mathbf{q}_n, \mathbf{q}_{n+1}) = \mathbf{D} \boldsymbol{\pi}(\mathbf{q}_{n+q})^T \bar{\nabla}_{\boldsymbol{\pi}} V(\boldsymbol{\pi}(\mathbf{q}_n), \boldsymbol{\pi}(\mathbf{q}_{n+1})) \quad (4)$$

with

$$\bar{\nabla}_{\boldsymbol{\pi}} V(\boldsymbol{\pi}(\mathbf{q}_n), \boldsymbol{\pi}(\mathbf{q}_{n+1})) = \nabla_{\boldsymbol{\pi}} V(\boldsymbol{\pi}_{n+\frac{1}{2}}) + \frac{V(\boldsymbol{\pi}_{n+1}) - V(\boldsymbol{\pi}_n) - \nabla_{\boldsymbol{\pi}} V(\boldsymbol{\pi}_{n+\frac{1}{2}}) \cdot (\boldsymbol{\pi}_{n+1} - \boldsymbol{\pi}_n)}{\|\boldsymbol{\pi}_{n+1} - \boldsymbol{\pi}_n\|^2} (\boldsymbol{\pi}_{n+1} - \boldsymbol{\pi}_n) \quad (5)$$

using a reparametrisation with possible invariants $\boldsymbol{\pi}$, which have to be members of either the set \mathcal{S} or \mathcal{T}

$$\begin{aligned} \mathcal{S}(\mathbf{q}) &= \{\mathbf{q}_A \cdot \mathbf{q}_B, 1 \leq A \leq B \leq n_{\text{nodes}}\} \\ \mathcal{T}(\mathbf{q}) &= \{\det([\mathbf{q}_A, \mathbf{q}_B]), 1 \leq A \leq B \leq n_{\text{nodes}}\} \end{aligned} \quad (6)$$

Possible invariants of the strain energy function can be identified as the components of the deformation tensor \mathbf{C} . The same approach can be applied to the contact constraint functions, as shown next.

* e-mail: hesch@imr.mb.uni-siegen.de, Phone: +49 271 740 2101, Fax: +49 271 740 2436

** e-mail: betsch@imr.mb.uni-siegen.de, Phone: +49 271 740 2224, Fax: +49 271 740 2436

2 Mortar method

The reparametrisation of the mortar constraints can be carried out with at least five invariants, three out of \mathcal{S} and two out of \mathcal{T}

$$\begin{aligned}
 \pi_1(\mathbf{q}_{\text{seg}}) &= (\mathbf{x}_2^{(1)} - \mathbf{x}_1^{(1)}) \cdot (\mathbf{x}_2^{(1)} - \mathbf{x}_1^{(1)}) \\
 \pi_2(\mathbf{q}_{\text{seg}}) &= (\mathbf{x}_2^{(1)} - \mathbf{x}_1^{(1)}) \cdot (\mathbf{x}_1^{(2)} - \mathbf{x}_1^{(1)}) \\
 \pi_3(\mathbf{q}_{\text{seg}}) &= (\mathbf{x}_2^{(1)} - \mathbf{x}_1^{(1)}) \cdot (\mathbf{x}_2^{(2)} - \mathbf{x}_1^{(1)}) \\
 \pi_4(\mathbf{q}_{\text{seg}}) &= (\mathbf{x}_2^{(1)} - \mathbf{x}_1^{(1)}) \cdot \Lambda(-2\mathbf{x}_1^{(1)} + \mathbf{x}_1^{(2)} + \mathbf{x}_2^{(2)}) \\
 \pi_5(\mathbf{q}_{\text{seg}}) &= (\mathbf{x}_2^{(1)} - \mathbf{x}_1^{(1)}) \cdot \Lambda(\mathbf{x}_1^{(2)} - \mathbf{x}_2^{(2)})
 \end{aligned} \tag{7}$$

where $\mathbf{x}_A^{(B)}$ denotes the four element nodes, defining a mortar segment (see Betsch & Hesch [1]) and Λ is a constant skew-symmetric matrix with

$$\Lambda = \begin{bmatrix} 0 & 1 \\ -1 & 0 \end{bmatrix} \tag{8}$$

A straightforward calculation shows, that the contact constraint functions can be recast as

$$\Phi_1^{\text{seg}}(\boldsymbol{\pi}(\mathbf{q}_{\text{seg}})) = \frac{1}{16}(\xi_b^{(1)} - \xi_a^{(1)}) \left\{ \pi_4 \int_{-1}^1 (\xi^{(1)} - 1) d\eta + \pi_5 \int_{-1}^1 (\xi^{(2)} - \xi^{(1)}\xi^{(2)}) d\eta \right\} \tag{9}$$

and

$$\Phi_2^{\text{seg}}(\boldsymbol{\pi}(\mathbf{q}_{\text{seg}})) = \frac{1}{16}(\xi_b^{(1)} - \xi_a^{(1)}) \left\{ \pi_5 \int_{-1}^1 (\xi^{(2)} + \xi^{(1)}\xi^{(2)}) d\eta - \pi_4 \int_{-1}^1 (\xi^{(1)} + 1) d\eta \right\} \tag{10}$$

again defined for one specific mortar segment, restricted by the local parametrisation ξ . A similar approach can be applied to the node-to-segment contact formulation (see Betsch & Hesch [2]).

3 Numerical example

The numerical example deals with the planar model of a bearing depicted in Fig. 1. The bearing consists of two rings (Young's modulus $E = 10^5$, Poisson's ratio $\nu = 0.1$ and mass density $\rho_R = 0.001$), which are discretized by 4-node isoparametric displacement-based plain strain elements. The discretization of the outer ring relies on 10x48 elements, for the inner ring 10x40 have been used.

For $t \in [0, 0.5]$, a torque acts on the inner ring in form of a hat function over time. Then, for $t \in (0.5, 2]$, no external loads are acting on the bearing anymore. Fig. 2 shows that for $t \geq 0.5$ the present scheme does indeed conserve the total energy for the frictionless contact problem under consideration.

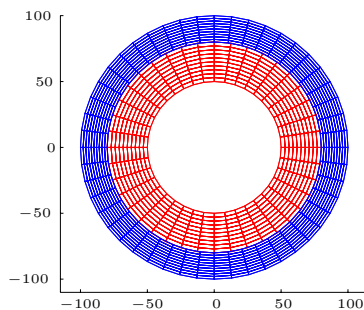


Fig. 1: Discretized bearing

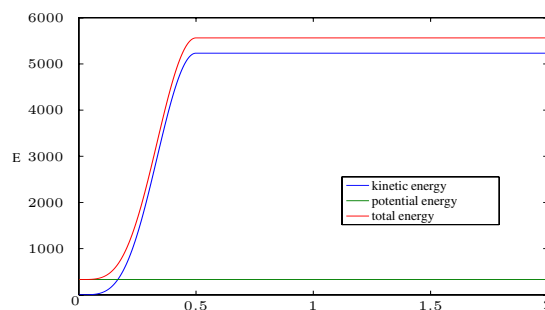


Fig. 2: Energy versus time

Literatur

- [1] C. Hesch and P. Betsch, On the energy-momentum conserving integration of large deformation contact problems. *In Proceedings of the ECCOMAS Thematic Conference*, Mailand, 25–28.06.2007
- [2] P. Betsch and C. Hesch, Energy-momentum conserving schemes for frictionless dynamic contact problems. Part I: NTS method *IUTAM Bookseries*, Springer-Verlag, 3:77–96, 2007.